

# PLANS OF PROPOSED P.P.C.C. BRIDGE OVER ON

**LENGTH** 48 400 OUT TO OUT OF ABUTMENT PRECAST BACKWALL PANELS

**SUPERSTRUCTURE** FOUR SIMPLY SUPPORTED SPANS OF PRECAST PRESTRESSED CONCRETE CHANNEL GIRDERS WITH ASPHALT OVERLAY

**SUBSTRUCTURE** TWO PRECAST CONCRETE ABUTMENTS AND THREE INTERMEDIATE BENTS WITH STEEL H-PILES

**ROADWAY WIDTH** 9 600 OUT TO OUT OF GIRDERS

**LOCATION** IN R.M. OF

## SHEET LEGEND

1. COVER SHEET
2. GENERAL ELEVATION
3. BORING LOGS
4. SITE AND EROSION CONTROL DETAILS
5. ASSEMBLY DETAILS
6. ASSEMBLY DETAILS
7. ASSEMBLY DETAILS
8. STEEL PILE CAP DETAILS
9. STEEL PILE CAP DETAILS
10. BEARING AND ERECTION DETAILS
11. RAILING LAYOUT AND DETAILS
12. RAILING DETAILS
13. RAILPOST DETAILS
  
- P1. PRECAST PANEL DETAILS
- P2. PRECAST PANEL DETAILS
  
- G1. PRECAST PRESTRESSED CHANNEL GIRDER DETAILS
- G2. PRECAST PRESTRESSED CHANNEL GIRDER DETAILS
- G3. PRECAST PRESTRESSED CHANNEL GIRDER DETAILS
- G4. PRECAST PRESTRESSED CHANNEL GIRDER DETAILS
- G5. PRECAST PRESTRESSED CHANNEL GIRDER DETAILS

## DESIGN DATA

### SPECIFICATIONS

AASHTO LRFD Bridge Design Specifications, First Edition, 1994 plus 1996/97 Interims

### VEHICULAR LIVE LOADING

1. Modified AASHTO HSS-25 Truck
2. AASHTO LRFD "HL-93" Loading

### STRUCTURAL CONCRETE

CSA A23.1, Exposure Class C-1 Air content category 1

1. PRECAST PRESTRESSED CONCRETE CHANNEL GIRDERS -  $f_c = 45 \text{ MPa}$  at 28 days  
 $f_{ci} = 35 \text{ MPa}$  at time of de-stressing
2. PRECAST PANELS -  $f_c = 35 \text{ MPa}$

### REINFORCING STEEL

1. PRECAST PRESTRESSED CONCRETE CHANNEL GIRDERS - CAN/CSA-G30.18-M92 Grade 400W black (i.e. no epoxy coating)
2. PRECAST PANELS - CAN/CSA-G30.18-M92 Grade 400W black (i.e. no epoxy coating)



### STRUCTURAL STEEL

1. All Structural Steel shall conform to CAN/CSA G40.21-M92 Grade 300W
2. HSS Tubing for Bridge Rail shall conform to CAN/CSA- G40.21-M92 Grade 350W

### PRESTRESSING STRAND

20-13 # low relaxation strands,  $f_{pu} = 1860 \text{ MPa}$

### PILE LOADING

MAXIMUM FACTORED LOAD	END PILE BENTS	INTERMEDIATE PILE BENTS
FACTORED BEARING RESISTANCE	 kN	 kN

## HYDRAULIC DESIGN DATA

### DESIGN DISCHARGE

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## SURVEY CONTROL

HORIZONTAL DATUM: NAD83CSRS

VERTICAL DATUM: CGVD28

ELLIPSOID: GRS 1980

GEOID (HT2.0): -----

UTM: ZONE ----

SCALE FACTOR: -----

### SITE CONTROL POINT DATA

CONTROL POINT *-----	NORTHING: -----	-----
	EASTING: -----	-----
	ELEVATION: -----	-----
	DATE: -----	-----
CONTROL POINT *-----	NORTHING: -----	-----
	EASTING: -----	-----
	ELEVATION: -----	-----
	DATE: -----	-----
CONTROL POINT *-----	NORTHING: -----	-----
	EASTING: -----	-----
	ELEVATION: -----	-----
	DATE: -----	-----



TP. -

RGE. -

## LOCATION MAP

Not to Scale

# MANITOBA INFRASTRUCTURE

WATER MANAGEMENT AND STRUCTURES

RELEASED FOR CONSTRUCTION BY :

EXECUTIVE DIRECTOR OF STRUCTURES

DATE -----



## ENVIRONMENTAL APPROVALS

- |                          |   |
|--------------------------|---|
| <input type="checkbox"/> | MANITOBA ENVIRONMENT ACT LICENCE                      |
|                          | DATE : _____  |
|                          | FILE # : _____  |
| <input type="checkbox"/> | FISHERIES AND OCEANS CANADA - AUTHORIZATION OR REVIEW |
|                          | DATE : _____  |
|                          | FILE # : _____  |
| <input type="checkbox"/> | TRANSPORT CANADA - NAVIGATION ACT                     |
|                          | DATE : _____  |
|                          | FILE # : _____  |
| <input type="checkbox"/> | MANITOBA INFRASTRUCTURE ENVIRONMENTAL APPROVAL        |
|                          | DATE : _____  |
|                          | FILE # : _____  |
| <input type="checkbox"/> | ENVIRONMENTAL REVIEW COMPLETED                        |
|                          | DATE : _____  |
|                          | COMPLETED BY : _____                                  |

ALL DIMENSIONS ARE IN MILLIMETRES (mm) AND ALL ELEVATIONS AND STATIONS ARE IN METRES (m) UNLESS SHOWN OTHERWISE.

DRAWN BY:

DATE:

SHEET No. 1

CHECKED BY:

DATE:

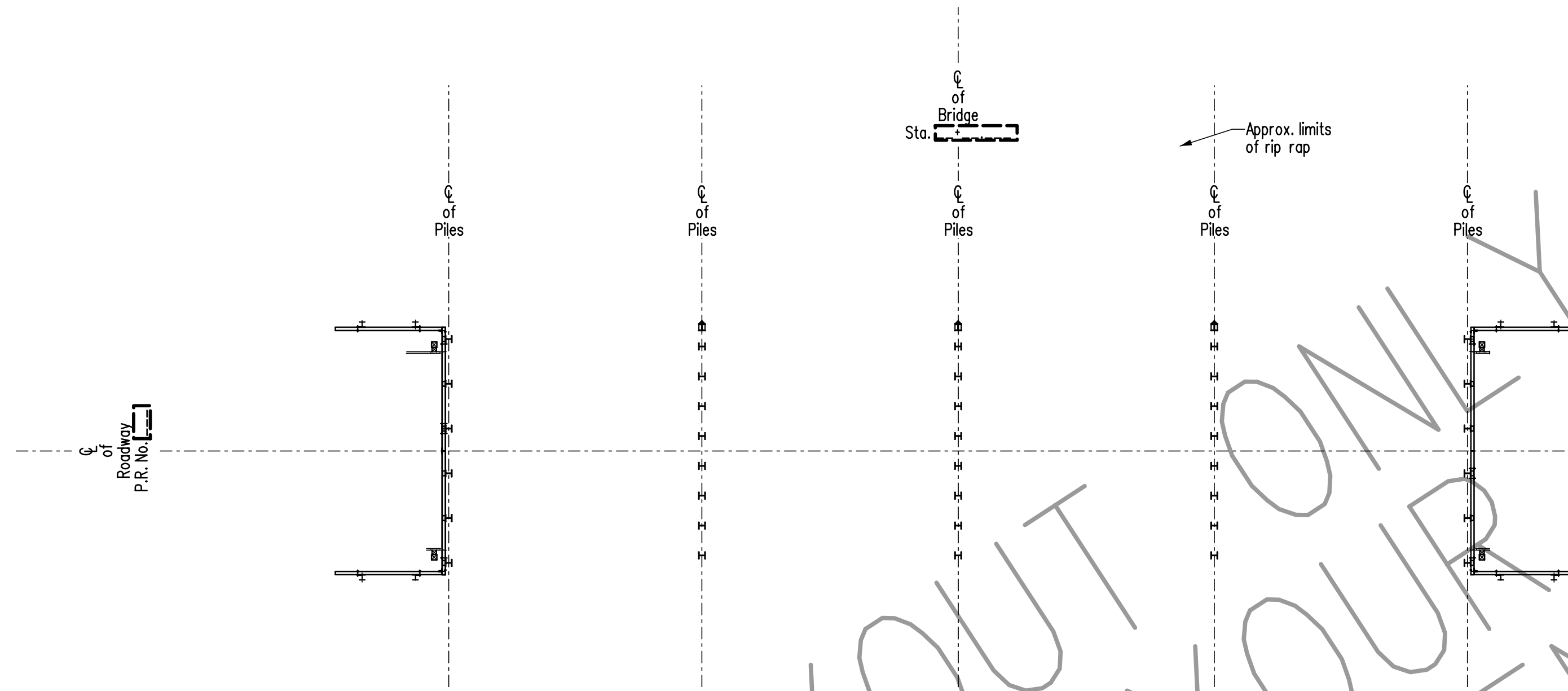
SITE No.



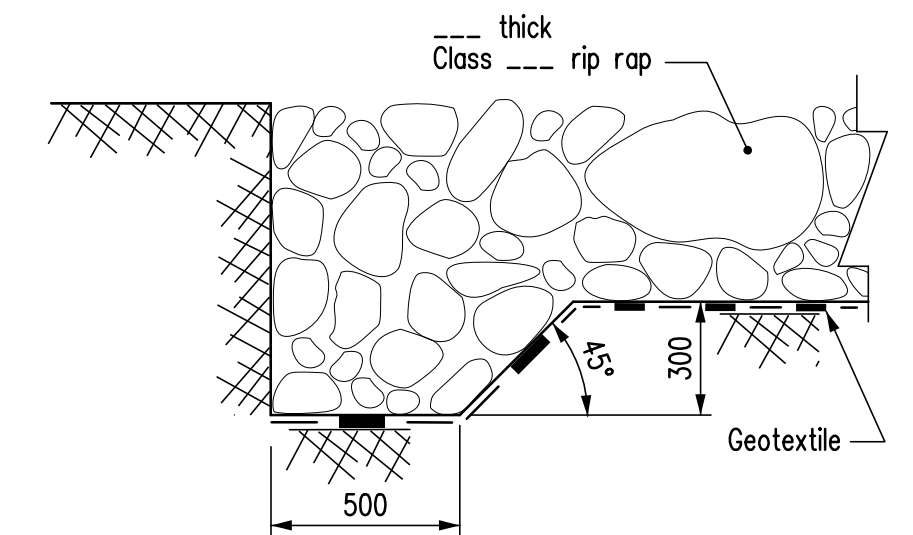


NORTH or WEST

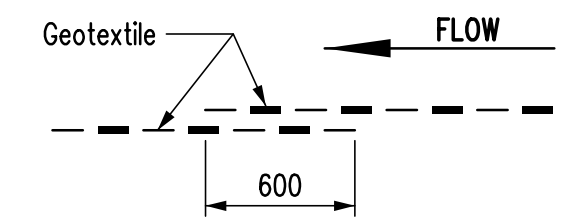
SOUTH or EAST



PLAN



EDGE TREATMENT



OVERLAPPING DETAILS

RIP RAP DETAILS

Not To Scale

NOTES:

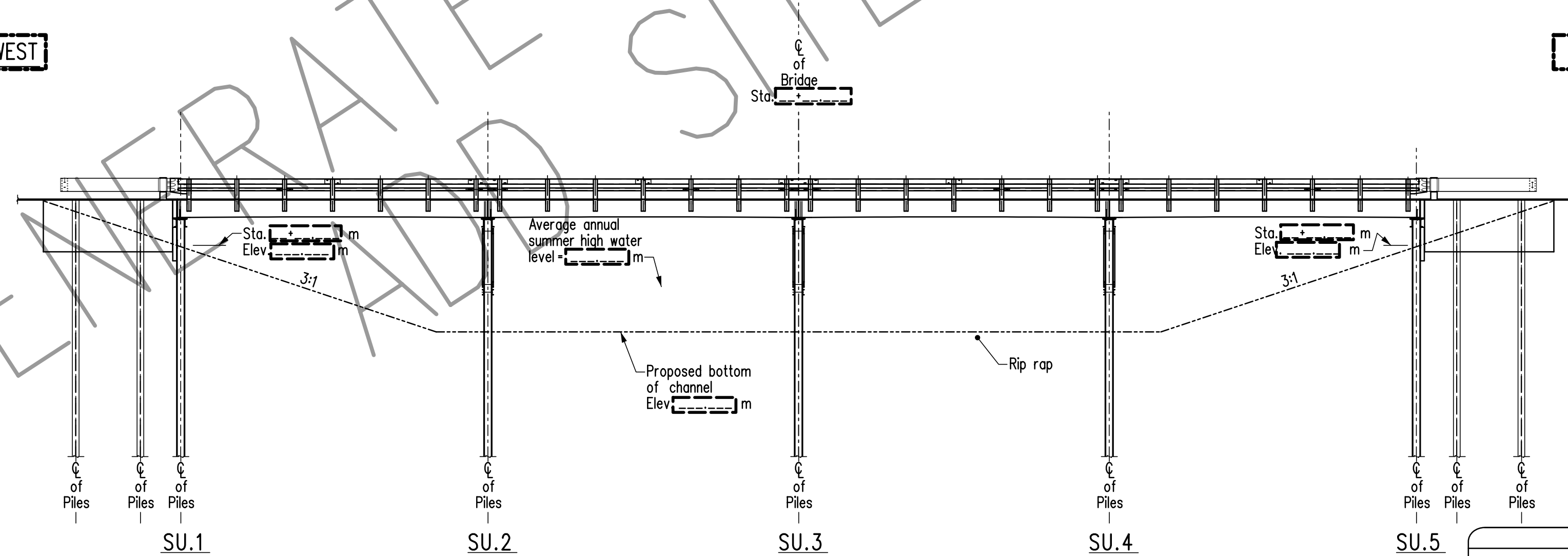
1. All geotextile shall be Non-Woven Geotextile, Class I (Heavy Duty) from the Manitoba Infrastructure's Approved Product List.
2. Geotextile shall be placed under all rip rap, overlapping 600mm in direction of flow.

NOTE:

Existing pile bents to be removed by Bridge Contractor.

NORTH or WEST

SOUTH or EAST



ELEVATION

Scale 1:150

**UTILITY DISCLAIMER:**  
 LOCATIONS OF UTILITIES AS SHOWN ARE BASED ON READILY AVAILABLE INFORMATION. NO GUARANTEE IS GIVEN THAT ALL UTILITIES ARE SHOWN OR THAT THE GIVEN LOCATIONS ARE EXACT. CONTRACTOR SHALL CONFIRM THE EXISTENCE AND LOCATION OF UTILITIES BY OBTAINING FROM THE INDIVIDUAL UTILITIES BEFORE PROCEEDING WITH CONSTRUCTION.

REVISIONS		SITE AND EROSION CONTROL DETAILS	
DATE	BY		
		RELEASED FOR CONSTRUCTION BY: _____ EXECUTIVE DIRECTOR OF STRUCTURES      DATE: _____	
		SCALE: 1:200 SHEET No. 4	
		or as shown      SITE No. _____	
		DESIGN BY: _____ CHECKED: _____	
		DETAILS BY: _____ CHECKED: _____	

PLACE ENGINEERS ELECTRONIC SEAL HERE







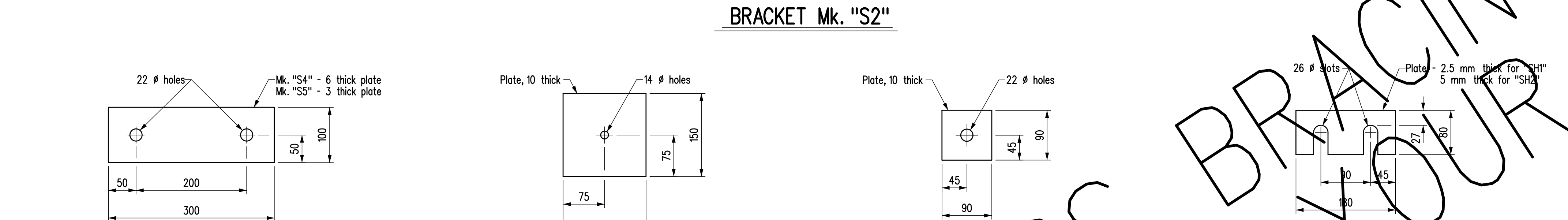
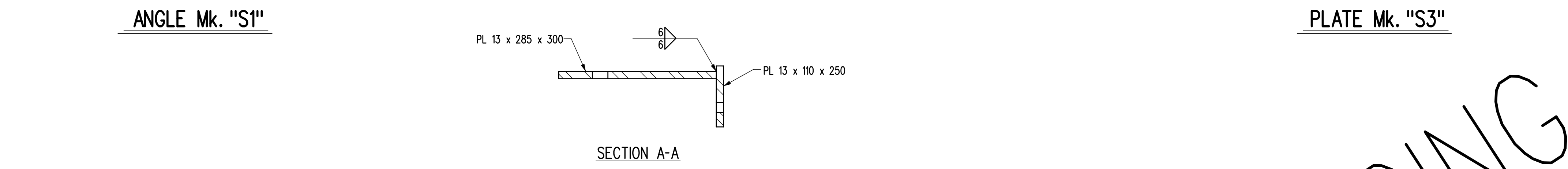
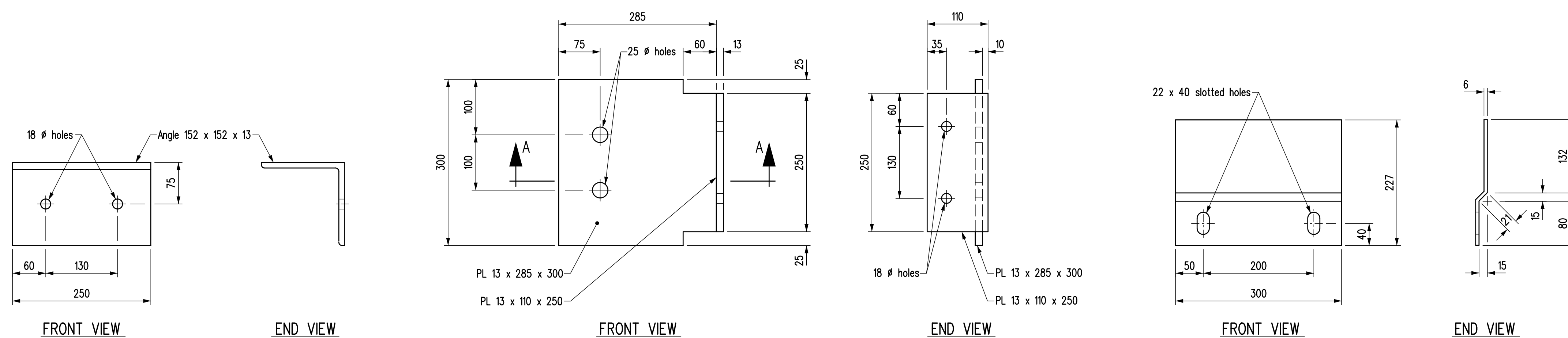




**BILL OF MISCELLANEOUS METAL 9 600 ROADWAY WIDTH - 4 SPAN - 0 DEGREE SKEW** Site No.

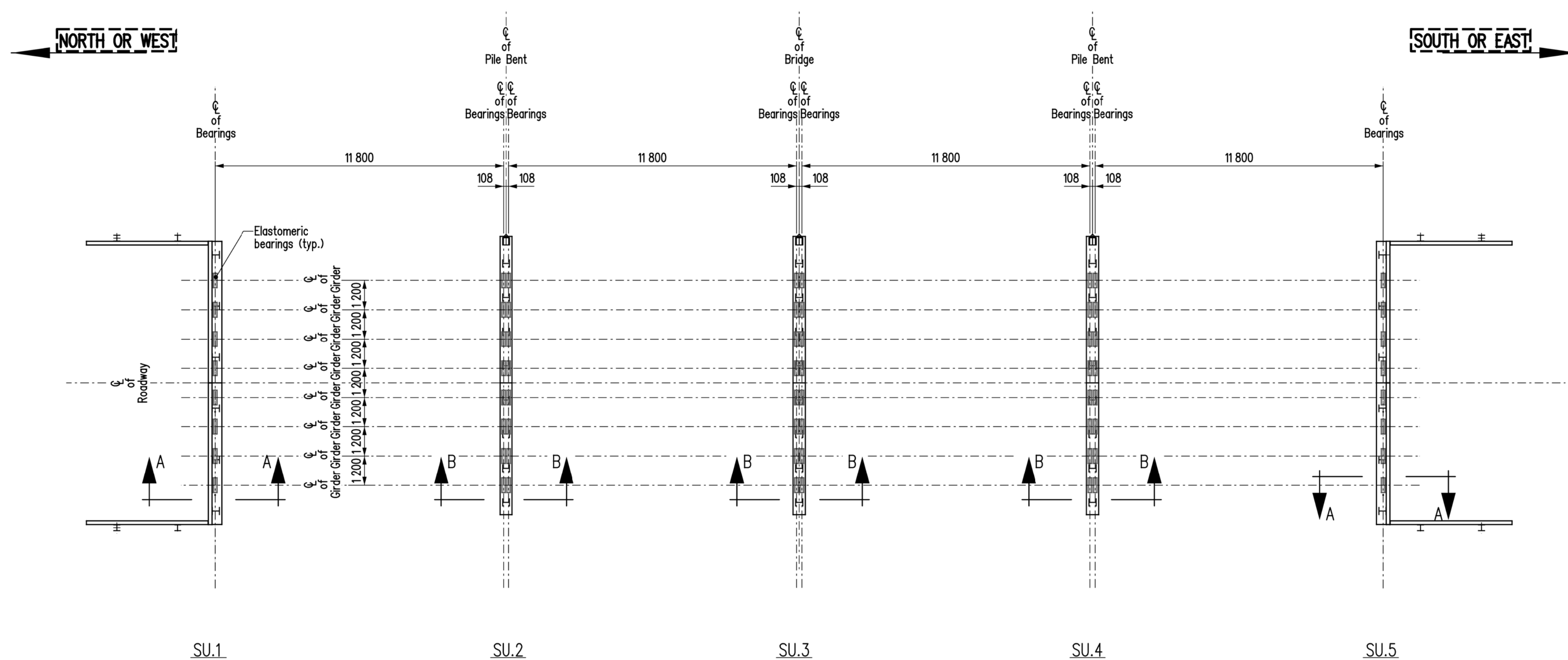
MARK No.	No.	DESCRIPTION	CORROSION PROTECTION	SIZE	LENGTH	REMARKS	COMPONENT MASS	MASS PER UNIT	TOTAL MASS
P1	2	Steel plate Each unit to be fabricated from: 1 - Steel plate 8 - Nelson Type NBL, no thread studs	Hot dip galvanized	PL 32x550 10 dia.	5 800 19	See detail for Abutment Part No. 101-063-167	801.328 0.012	801.328 0.096	1602.85
P1a	2	Steel plate Each unit to be fabricated from: 1 - Steel plate 8 - Nelson Type NBL, no thread studs	Hot dip galvanized	PL 32x550 10 dia.	5 800 19	See detail for Abutment Part No. 101-063-167	801.328 0.012	801.328 0.096	1602.85
P2	3	Steel plate Each unit to be fabricated from: 1 - Steel plate 16 - Nelson Type NBL, no thread studs	Hot dip galvanized	PL 32x550 10 dia.	5 400 19	See detail for Intermediate Bent Part No. 101-063-167	678.240 0.012	678.240 0.192	2035.30
P2a	3	Steel plate Each unit to be fabricated from: 1 - Steel plate 16 - Nelson Type NBL, no thread studs	Hot dip galvanized	PL 32x550 10 dia.	6 000 19	See detail for Intermediate Bent Part No. 101-063-167	753.600 0.012	753.600 0.192	2261.38
R1	4	Steel channel	Hot dip galvanized	C310x45	11 800	See detail for Abutment		518.520	2074.08
P4	3	Steel channel	Hot dip galvanized	C310x45	11 400	See detail for Intermediate Bent		509.580	1528.74
P4a	3	Steel channel	Hot dip galvanized	C310x45	11 400	See detail for Intermediate Bent		509.580	1528.74
R30	23	A325 bolt assembly	Hot dip galvanized	16 dia.	89	Steel plate to channels		0.245	53.41
R32	44	A325 bolt assembly	Hot dip galvanized	16 dia.	76	Steel plate to channels C bore holes		0.225	9.90
R35	468	A325 bolt assembly	Hot dip galvanized	22 dia.	64	Channels to piles		0.461	215.75
R36	48	A325 bolt assembly	Hot dip galvanized	16 dia.	64	Angles Mk. "S1" to piles & bracket Mk. "S2" to cap		0.205	9.84
S1	20	Angle	Hot dip galvanized	L 152x152x13	250	As detailed		7.250	145.00
S2	4	Bracket	Hot dip galvanized			As detailed		11.226	44.90
S3	16	Plate	Hot dip galvanized	PL 6x300		As detailed		3.223	51.57
S4	32	Flange plate	Hot dip galvanized	PL 6x100	300	As detailed		1.413	45.22
S5	16	Flange plate	Hot dip galvanized	PL 3x100	300	As detailed		0.707	11.31
A1	16	Structural plate w/asher	Hot dip galvanized	PL 10x150	150	As detailed - One to threaded rod Mk. "TR2"		1.766	28.26
A2	4	Structural plate w/asher	Hot dip galvanized	PL 10x90	90	As detailed - One to bolt Mk. "R34"		0.636	5.09
TR1	64	Threaded rods c/w w/o hex. nuts	Hot dip galvanized	19 dia.	400	Girder to steel cap plate		0.940	60.16
TR3	32	Threaded rods c/w w/o hex. nuts	Hot dip galvanized	19 dia.	300	Steel plates Mk. "S3" to precast panels		0.660	21.12
W1	16	Hardened bevel w/asher	Hot dip galvanized	for 16 dia. bolts		One to bolts Mk. "R30" & "R32"		0.110	28.82
W2	128	Standard flat w/asher	Hot dip galvanized	for 13 dia. rod		One to threaded rod Mk. "TR2"		0.010	0.16
W3	16	Standard flat w/asher	Hot dip galvanized	for 19 dia. rod		One to "TR1", two to "TR3"		0.020	2.56
W4	16	Structural lock w/asher	Hot dip galvanized	for 12 dia. rod		One to threaded rod Mk. "TR2"		0.010	0.16
W5	96	Structural lock w/asher	Hot dip galvanized	for 19 dia. rod		One to "TR1" & "TR3"		0.020	1.92
W6	468	F436 Hardened w/asher	Hot dip galvanized	for 22 dia. bolts		One to bolt Mk. "R35"		0.032	14.98
W7	48	F436 Hardened w/asher	Hot dip galvanized	for 16 dia. bolts		One to bolt Mk. "R36"		0.014	0.67
R1	224	A325 bolt assembly	Hot dip galvanized	22 dia.	76	R.C. girder connection		0.499	111.78
W1	224	Structural flat w/asher	Hot dip galvanized	for 22 dia. bolts		One to bolt Mk. "R1"		0.050	11.20
W2	224	Pair Nurd-Lock lock w/ashers	Hot dip galvanized	for 22 dia. bolts		One pair to bolt Mk. "R1"		0.020	4.48
SH1	84	Shim plate	Hot dip galvanized	PL 2.5x80	180	As detailed - use as required		0.231	19.40
SH2	84	Shim plate	Hot dip galvanized	PL 5x80	180	As detailed - use as required		0.463	38.89
IP1	6	Plate	Shop Primed	PL277x20	350	See Ice Breaker Details		15.221	91.33
IP2	6	Plate	Shop Primed	PL277x20	500	See Ice Breaker Details		21.745	0.00
IU1	3	Ice Breaker Unit	Shop Primed						7.04
		Each unit fabricated from:							
		1 - Angle		L203x203x13	0	As detailed		0	0.000
		- Stiffener Steel Plate		100x13	230	Fitted stiffeners as detailed	2.347	2.347	2.347
CB1	6	Channel	Shop Primed	C200x21	0			0.000	0.00
CB2	12	Channel	Shop Primed	C200x21	0			0.000	0.00
<b>TOTAL MASS (kg) = 13668.84</b>									

**NOTES:**  
 1. All material noted in the above Bill shall be hot dip galvanized after fabrication in accordance with CSA G164 for a minimum net retention of 610 g/m<sup>2</sup> unless otherwise stated in the specified material ASTM standards. The fabricator and galvanizer shall safeguard against embrittlement using recommended practices from applicable standards.  
 2. Seal all welds prior to galvanizing.  
 3. Apply Galvaloy to all field welds and areas where galvanizing has been damaged.  
 4. All bolts and threaded rod in the above Bill shall be Imperial thread.



ICE BREAKER & CROSS BRACING SHOULD BE CHANGED TO YOUR PREFERENCE

REVISIONS		STEEL PILE CAP DETAILS	
DATE	BY	DESCRIPTION	RELEASED FOR CONSTRUCTION BY:
DESIGN SEAL	RECORD SEAL		
PLACE ENGINEERS ELECTRONIC SEAL HERE		Manito Infrastructure Water Management and Structures	
DESIGN	CHECKED:	BY:	EXECUTIVE DIRECTOR OF STRUCTURES DATE
DETAILS	CHECKED:	BY:	SCALE: 1:5 SHEET No. 10
			or as shown SITE No.



SU.1 SU.2 SU.3 SU.4 SU.5  
**PLAN**

BILL OF BEARINGS			9 600 ROADWAY WIDTH - 4 SPAN	Site No.
No.	LOCATION	DESCRIPTION	REMARKS	
64	SU.1 - SU.5	Elastomeric bearings	As detailed	

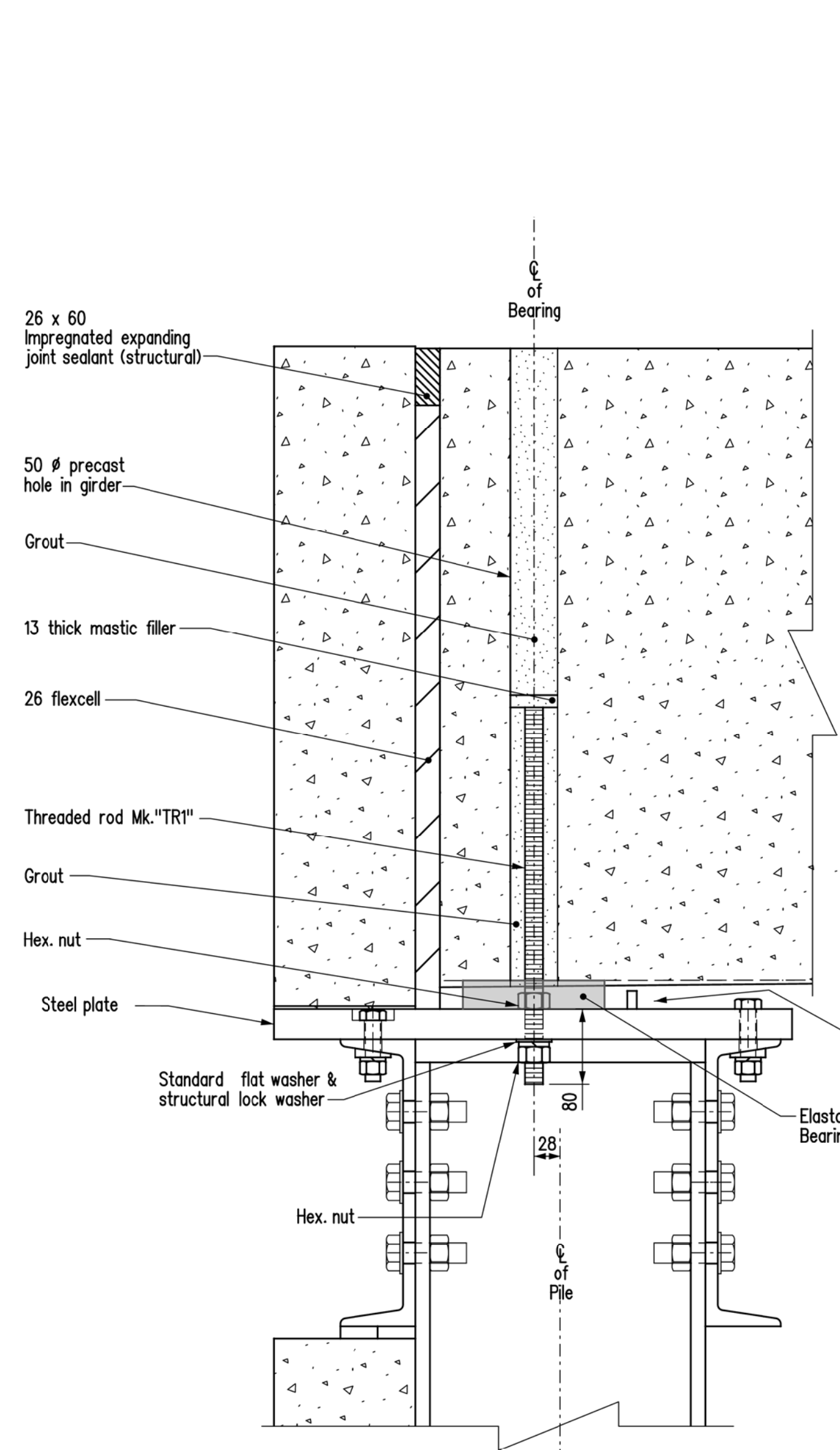
**NOTE:**

- Elastomer shall be natural rubber. Elastomer shall be AASHTO low temperature Grade 5 with a minimum shear modulus  $G \geq 0.9$  MPa and a 60 durometer Shore A hardness.
- Internal steel reinforcing plates for laminate bearings shall be rolled mild steel with a minimum yield strength of 300 Mpa.

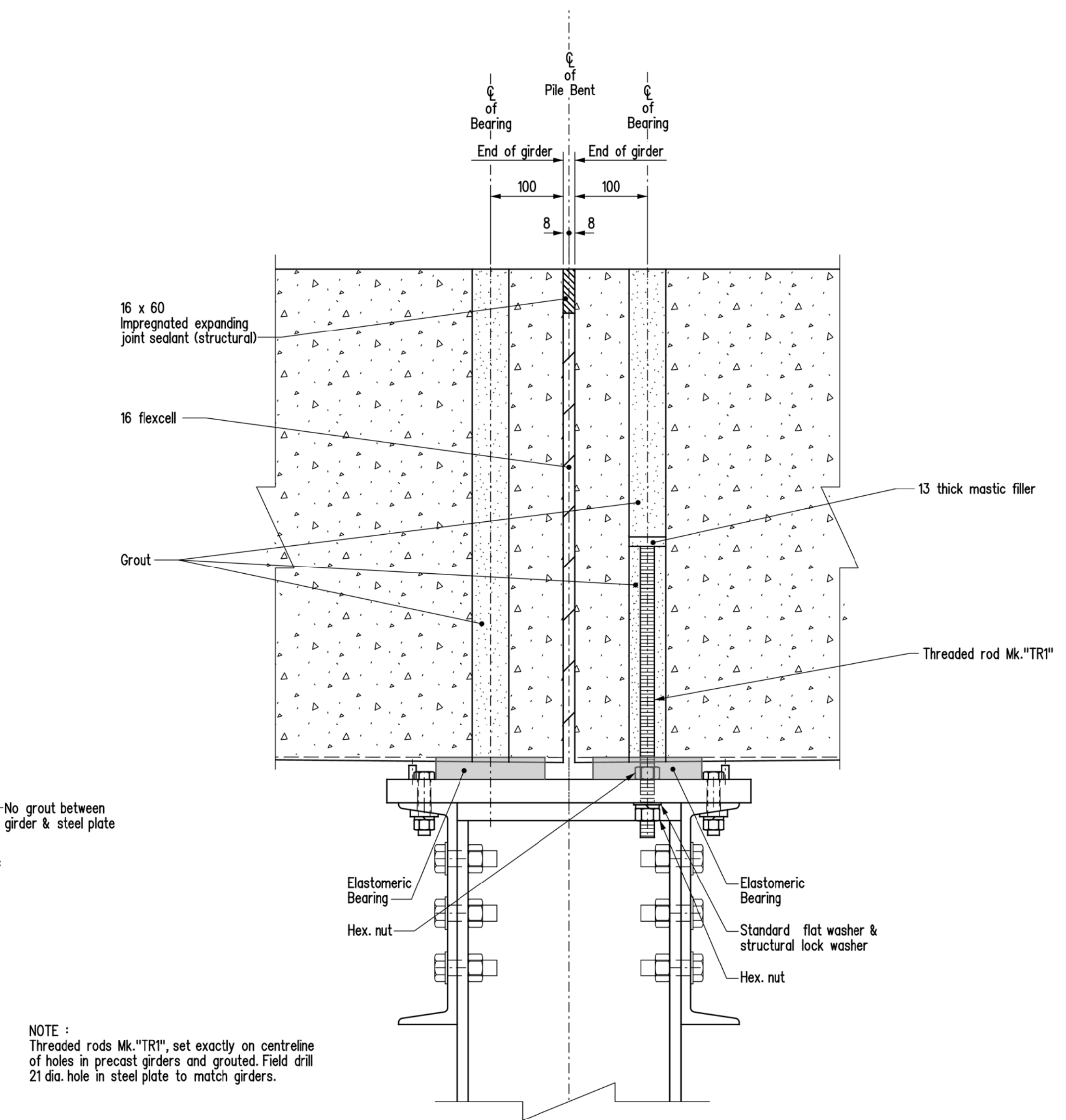
PLAN  
Scale 1:10

PART CROSS SECTION  
Scale 1:2

**ELASTOMERIC BEARINGS**



**SECTION "A-A"**  
Threaded rods at SU.1 & SU.5. See sheet No. 6 for layout.  
Scale 1:5



**SECTION "B-B"**  
Threaded rods at SU.2, SU.3 & SU.4. See Sheet No. 6 for layout.  
Scale 1:5

**NOTE:**  
Threaded rods Mk. "TR1" set exactly on centreline of holes in precast girders and grouted. Field drill 21 dia. hole in steel plate to match girders.

- NOTES:**
- Re: Girder Erection Operations Behind Abutment Ballast Walls
- Surcharge loading on the backfill resulting from girder erection operations shall be minimized near the precast concrete ballast walls and wingwalls.
  - Where possible, girder erection equipment shall be positioned such that there are no surcharge loads behind the back face of the precast panels within a distance equal to the depth of backfill to the bottom of the panels at the time of girder erection.
  - Should the Contractor propose to encroach on this zone, the following requirements must be satisfied:
    - Submit a girder erection procedure for approval outlining type, configuration, weights and locations of equipment including expected tipping forces on crane outriggers, etc.
    - Perform all precautionary measures outlined by the Department as a result of that submission.
    - All surcharge loads encroaching in this zone must be distributed over an area not less than 2.0 m<sup>2</sup>.

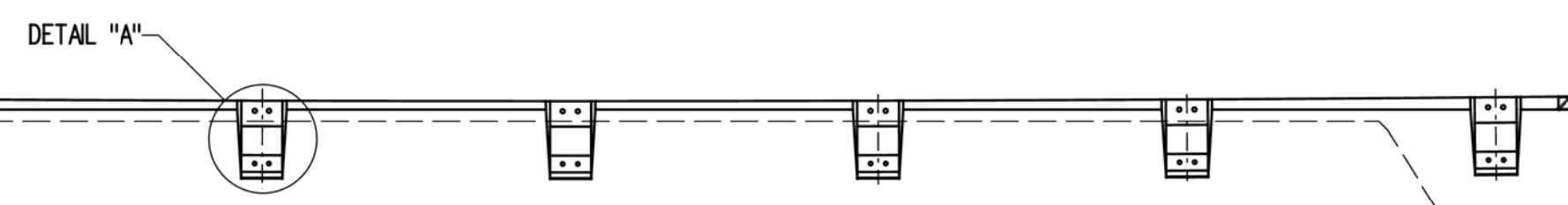
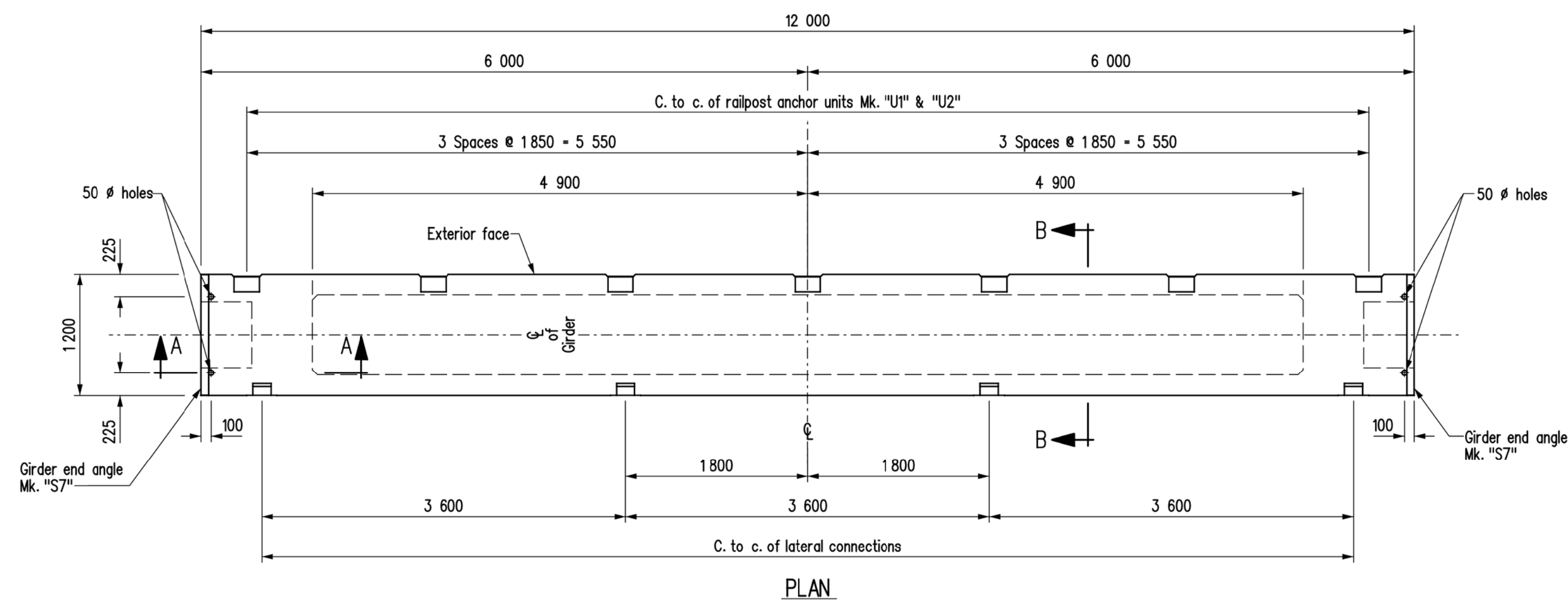
REVISIONS		BEARING AND ERECTION DETAILS	
DATE	BY		
		<p>Infrastructure Water Management and Structures</p>	
		<p>RELEASED FOR CONSTRUCTION BY:</p>	
		<p>EXECUTIVE DIRECTOR OF STRUCTURES DATE</p>	
		<p>DESIGN BY: _____</p> <p>CHECKED: _____</p>	<p>SCALE: 1:125</p> <p>SHEET No. 11</p>
		<p>DETAILS BY: _____</p> <p>CHECKED: _____</p>	<p>or as shown SITE No. _____</p>

PLACE ENGINEERS ELECTRONIC SEAL HERE

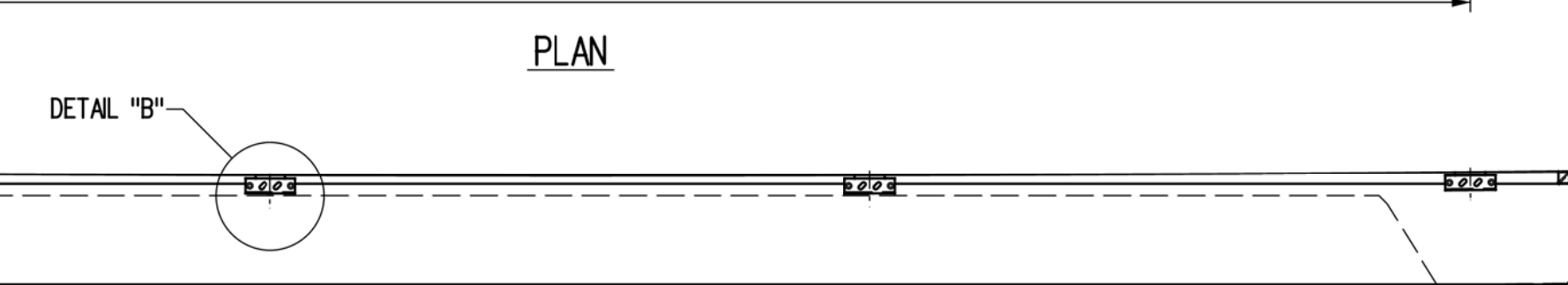
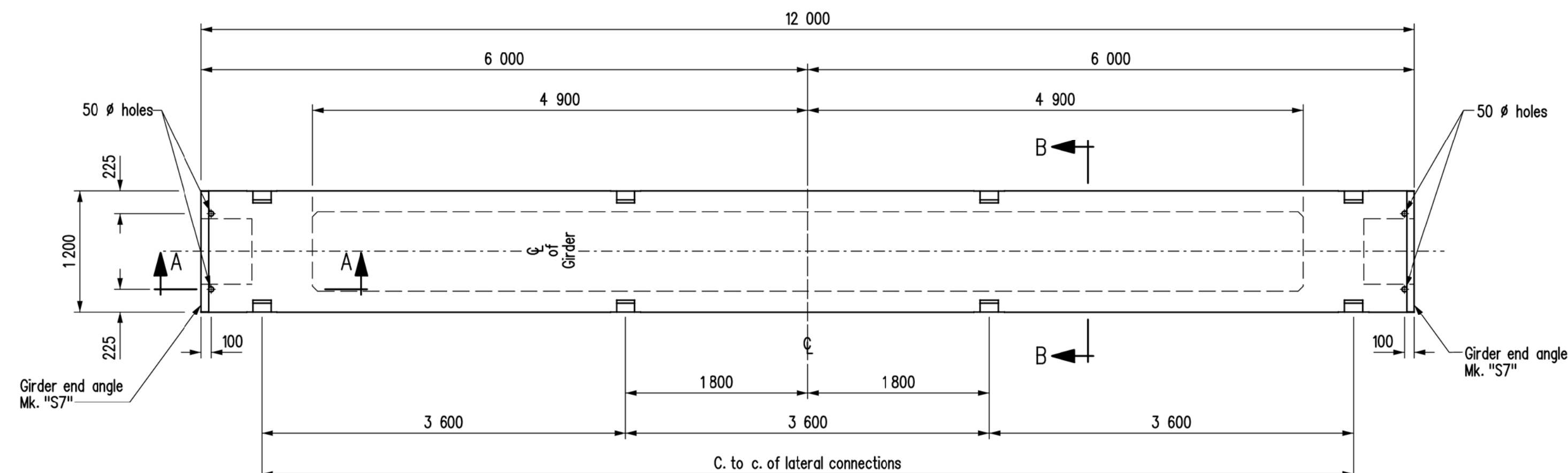




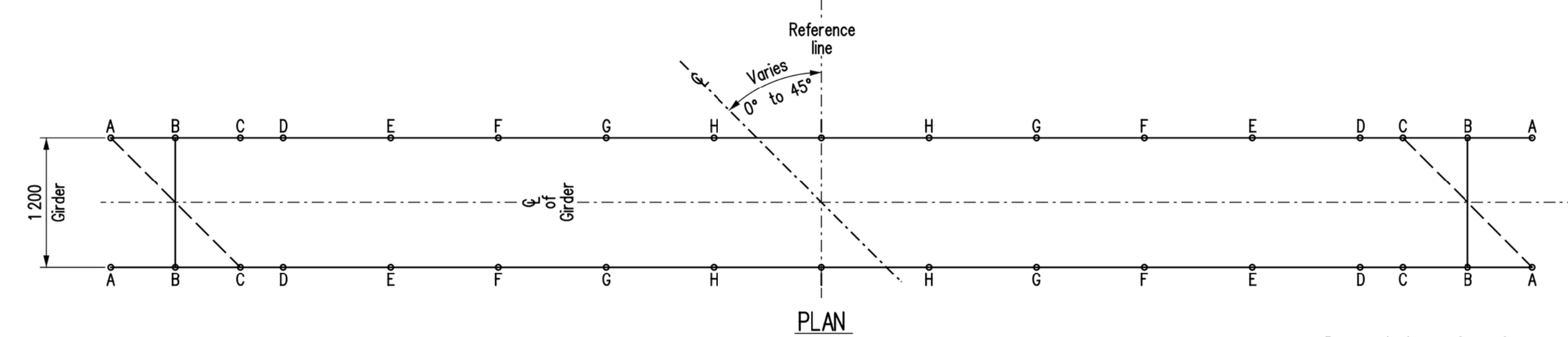




EXTERIOR GIRDER MK. "G1"



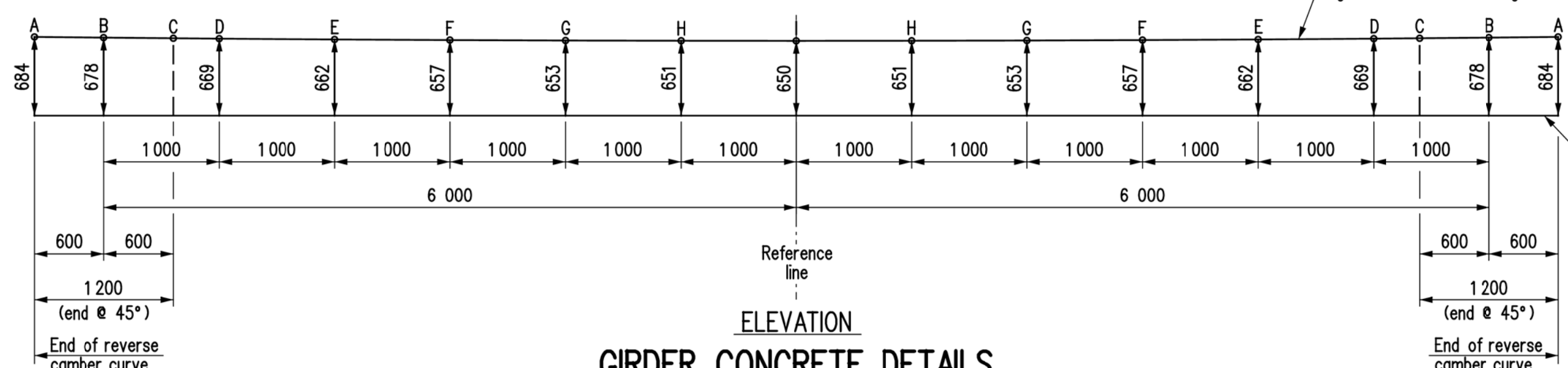
INTERIOR GIRDER MK. "G2"



NOTE: Top surface of girder shall be screeded perpendicular to side forms

Represents top surface of girder before distressing

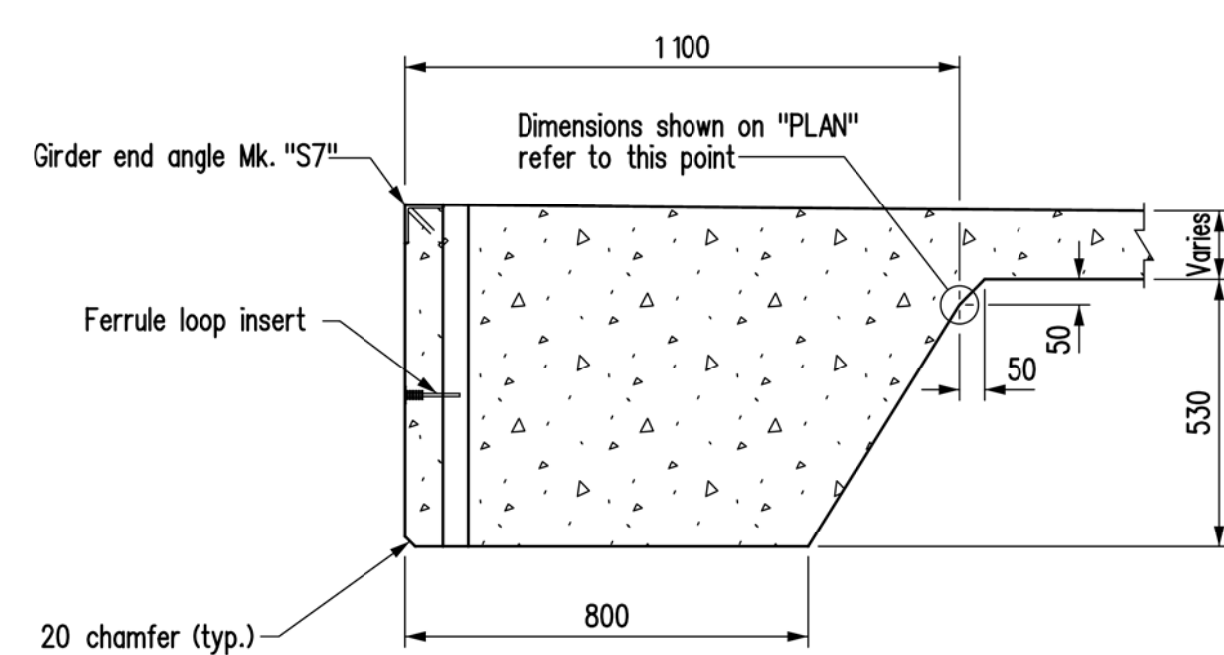
Represents bottom surface of girder before distressing (level)



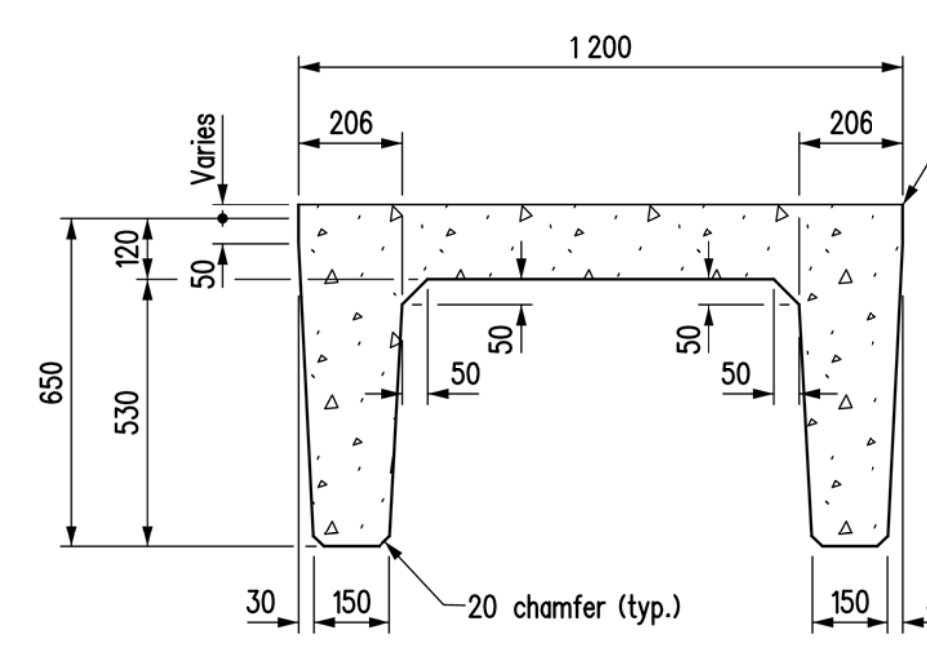
GIRDER CONCRETE DETAILS

Showing variable depth of girder to eliminate camber on top surface after distressing

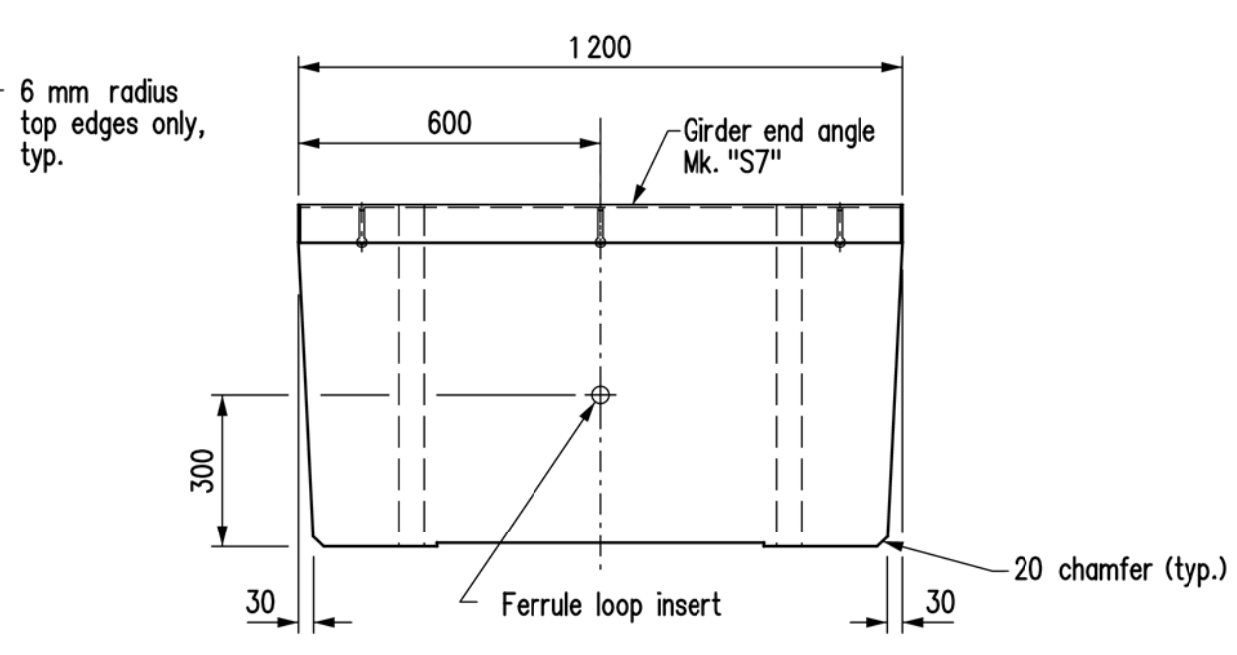
NOTE: The end of girder will fall between POINT "A" and POINT "C" on curve because of various skew angles.



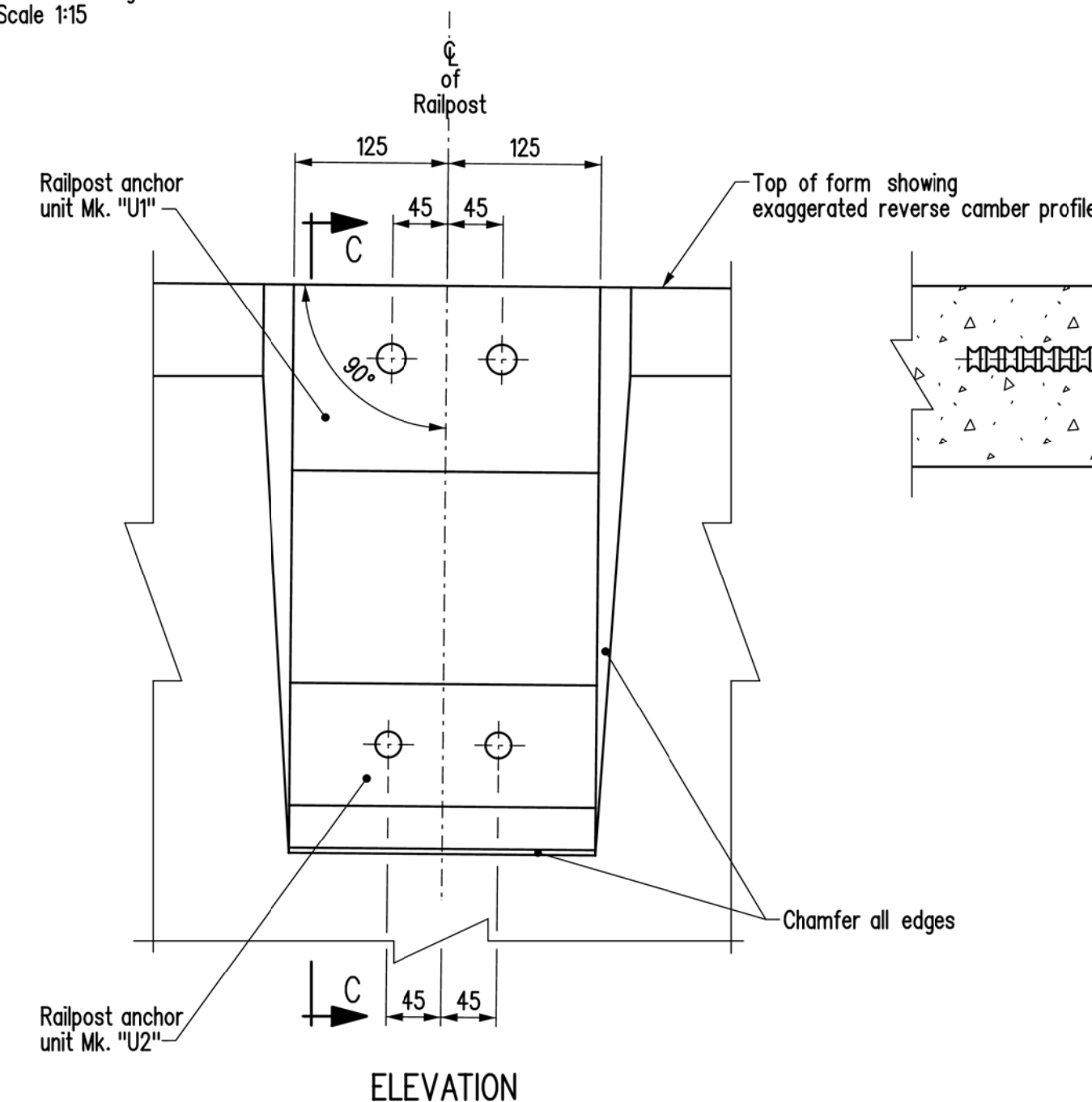
SECTION A-A  
Typical at both ends of girders  
Scale 1:15



SECTION B-B  
Scale 1:15



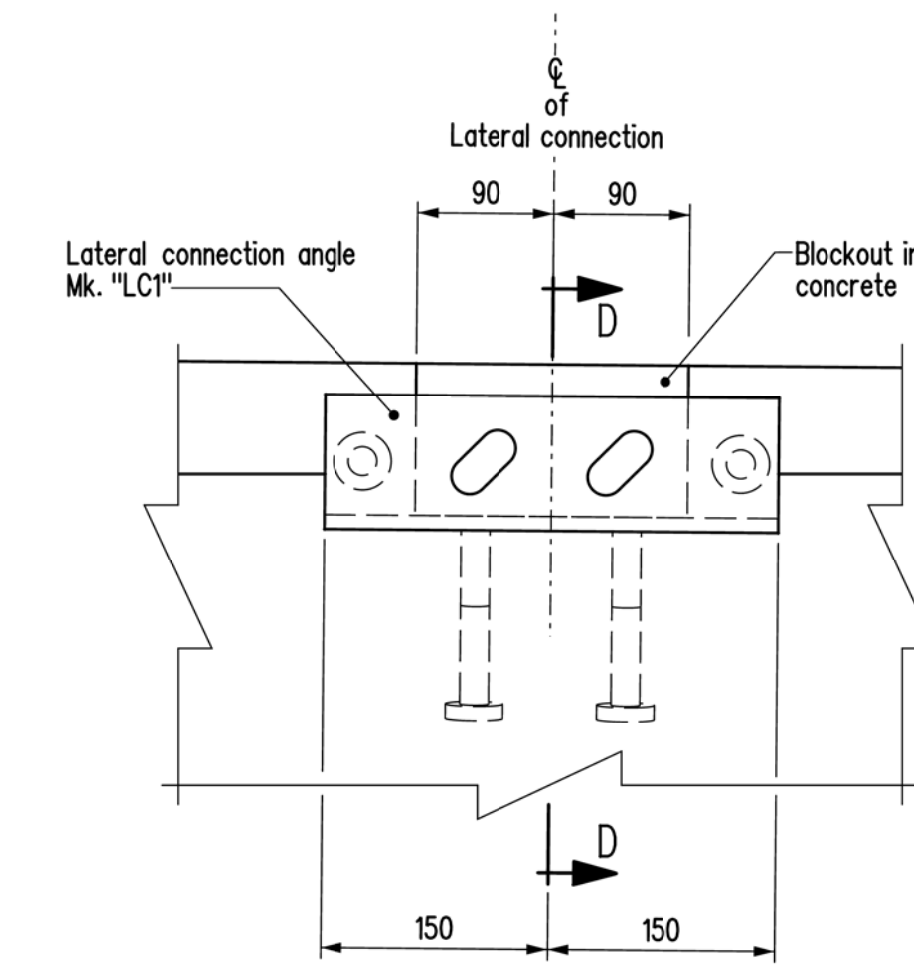
END VIEW  
Scale 1:15



ELEVATION

DETAIL "A"  
Scale 1:5

SECTION C-C



ELEVATION

DETAIL "B"  
Scale 1:5

SECTION D-D

NOTES:

- Design in accordance with AASHTO LRFD Bridge Design Specifications, First Edition, 1994 plus 1996/1997 interims.
- Design Vehicular Live Load: Modified AASHTO HSS-25 AASHTO LRFD "HL-93"
- Design distribution factor = 0.5 lanes/girder.
- Concrete strength:  $f_{ci}$  transfer,  $f_{ci} = 35$  MPa  
 $f_c$  28 days,  $f_c = 45$  MPa
- Prestressing steel: 13 mm  $\phi$  low relaxation strands  
Minimum ultimate strength,  $f_{pu} = 1860$  MPa  
Jacking force/strand,  $f_{pj} = 128.5$  kN/strand
- Girder dimensioning tolerances: Length 3 mm  $\pm$   
Cross section 2 mm  $\pm$
- Approximate mass per girder = 12,000 kg

REVISIONS		PRECAST PRESTRESSED CHANNEL GIRDER DETAILS	
DATE	BY	DESIGN SEAL	RECORD SEAL
PLACE ENGINEERS ELECTRONIC SEAL HERE			
DESIGN	BY: _____	EXECUTIVE DIRECTOR OF STRUCTURES	DATE
DETAILS	CHECKED: _____	SCALE: Scale 1:40	SHEET No. G1
	CHECKED: _____	or as shown	SITE No. _____













